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The Diffusion of Transverse Loads in a Reinforced Circular Cylinder with Non-rigid Frames

By

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of The College of Aeronautics

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The Diffusion of Transverse Loads in a Reinforced Circular Cylinder with Non-Rigid Frames

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Summary

This report is based upon the work of W.J. Goodey(1). It explores the variation of shear stress distribution at a loaded frame in a reinforced circular cylinder, consequent upon the variation of several geometrical parameters such as frame stiffness, skin thickness, stringer spacing, etc. The results are given in Figures 5-17 which indicate the shear distribution around a frame for a single concentrated radial load of 1000 lb. The parameters chosen are those common to aircraft design, and it is possible to obtain a reasonably accurate shear distribution around a frame from the data supplied, without doing the actual lengthy shear calculation.

The work is confined to the particular case where the loaded frame is removed from the end of the cylinder (see Figure 1).

Indication is also given as to when the beam theory distribution of shear can be used with a reasonable degree of accuracy.

The Appendix gives the method of obtaining the shear load due to a tangential load and moment from the radial load expressions.

FIGURE 1.
NOTATION

The notation used is:

$R$ = the radius of the frame at the skin line, ins.
$t$ = the actual skin thickness, ins.
$J_1$ = the moment of inertia of the adjacent frame, about an axis parallel to the skin line, ins.$^4$
$J_2$ = the moment of inertia of the loaded frame about an axis parallel to the skin line, ins.$^4$
$L$ = frame spacing, ins.
$\nu$ = Poisson's ratio, value taken as 0.3.
$N_x0$ = shear per ins in the skin.
$m = \frac{P}{L}$

$\psi = 3 m^2 \cdot 2 k_x (1 + \nu) - \nu$

$\phi = \frac{R^2 k_x t}{J_1}$

$c = 6 m^3 \phi$

$k_x$ = ratio of equivalent skin thickness to actual skin thickness, see fig. (2)

If the skin is not buckled $k_x = \frac{(A_3 + bt)}{bt}$

If the skin is buckled and the effective width of skin is $b'$ $k_x = \frac{(A_3 + b't)}{bt}$

Fig. (2)
Introduction

The problem considered is that of the shear distribution around a non-rigid frame of a reinforced circular cylinder, with the loaded frame at a distance away from the ends. The method of solution is obtained by superimposing the results of the two cases indicated below and shown in Figure 3.

Cases: 

(i) A long circular cylinder with the loaded frame at the middle,

(ii) A long circular cylinder with the loaded frame at the free end.

FIGURE 3

Solutions

The solutions contained in this report are for the cases (i) and (ii) above, and the expression for the shear in the skin at the loaded frame, due to a radially applied load $W$, is given by

\[ N_{x0} = \frac{W}{2\pi} \sum_{n=1}^{\infty} \frac{\left[ (\lambda_2+1)(\lambda_1-1)^2-(\lambda_1+1)(\lambda_2-1)^2 \right] n \sin n\theta}{(\lambda_2+1)(\lambda_1-1)^2(\lambda_1+2-r)-(\lambda_1+1)(\lambda_2-1)^2(\lambda_2+2-r)} + \frac{W}{2\pi} \sin \theta, \quad \ldots \ldots \quad (1) \]

or

\[ N_{x0} = \frac{W}{2\pi} \sum_{n=1}^{\infty} \frac{\left[ \frac{a^2 \sin 2\gamma + a(a^2-2) \sin \gamma - a \sin \gamma}{\sin \gamma \left\{ 2a^2(a^2+2a \cos \gamma - 3) \right\} - r(a^4-5a^2+1)-2ra(1+a^2) \cos \gamma} \right] n \sin n\theta}{(\lambda_2+1)(\lambda_1-1)^2(\lambda_1+2-r)-(\lambda_1+1)(\lambda_2-1)^2(\lambda_2+2-r)} + \frac{W}{2\pi} \sin \theta, \quad \ldots \ldots \quad (2) \]

The above expressions for $N_{x0}$ are given by Goodey where, for the loaded frame, $k=1$.

In expression (1) above $\lambda_1$ and $\lambda_2$ are obtained as shown below.

Solve the equation

\[ X^2 + UX + V = 0 \quad \ldots \ldots \quad (3) \]

where

\[ U = \left\{ \frac{n^2(n^2-1)^2(n^2-2)}{c^4} - 1 \right\}, \quad \ldots \ldots \quad (4) \]
\[ V = 4 \left\{ 1 + \frac{n^2(n^2 - 1)^2}{c^2} \left( \frac{n^2 + 3}{c^2} \right) \right\} \]  \hspace{1cm} (5)

The roots of equation (3) are \( X_1 \) and \( X_2 \)

Next solve the equations

\[ \lambda + \frac{1}{\lambda} = X_1 \quad \text{and} \quad \lambda + \frac{1}{\lambda} = X_2 \]  \hspace{1cm} (6)

The roots of equations (6) are \( \lambda_1 \) and \( \lambda_3 \), and \( \lambda_2 \) and \( \lambda_4 \) respectively, where \( \lambda_1 = \frac{1}{\lambda_3} \) and \( \lambda_2 = \frac{1}{\lambda_4} \).

The values of \( \lambda \) used in the expression (1) for \( N_{x \theta} \) are those less than unity.

If the roots \( X_1 \) and \( X_2 \) of equation (3) are less than 2.0 then we use expression (2) for \( N_{x \theta} \).

"a" and "\( \mu \)" are obtained as shown below.

\[ Y^2 - (V + 4)Y + U^2 = 0 \]  \hspace{1cm} (7)

where \( U \) and \( V \) are given by expressions 4 and 5.

The roots of equation (7) are \( Y_1 \) and \( Y_2 \) where one root is greater than 4.0 and one less than 4.0.

Let \( Y_1 \) be the root greater than 4.0 and \( Y_2 \) the root less than 4.0, we then solve the equations

\[ (a + 1) = \sqrt{Y_1} \quad \text{and} \quad (2 \cos \varphi)^2 = Y_2 \]  \hspace{1cm} (8)

The values of "a" found are \( a_1 \) and \( a_2 \) where \( a_1 = \frac{1}{a_2} \), and the value of "a" used in the expression (2) for \( N_{x \theta} \) is that which is less than unity.

The value of "\( \mu \)" used in the expression (2) for \( N_{x \theta} \) is the value given by \( Y_2 \) which is of opposite sign to "V" in expression (4). This is so, because

\[ U = 2(a + 1) \cos \varphi \]

the term \( (a + 1) \) is always +ve hence Cor \( \mu \) must be -ve, i.e. the sign of "\( \mu \)" must be opposite to that of "V".

The expression for \( N_{x \theta} \) can be written in the form

\[ N_{x \theta} = \sum_{\lambda=1}^{\infty} F(a, \lambda, k, r) n \sin n \theta + \frac{W}{2 \pi R} \sin 0 \]  \hspace{1cm} (9)

or

\[ N_{x \theta} = \sum_{\lambda=1}^{\infty} F(a, \mu, k, r) n \sin n \theta + \frac{W}{2 \pi R} \sin \theta \]  \hspace{1cm} (10)

In both expressions 9 and 10 the value of \( F(\text{---}) \) \( n \sin n \theta \) for \( n = 1 \) is equal to \( \frac{\sin \theta}{2} \).

For practical purposes it is sufficiently accurate to take the \( \sum \) term up to a value of \( n = 6 \), hence the expression for shear can be written as
\[ N_{x0} = \frac{W}{\pi R} \sin \theta + \frac{W}{\pi R} \sum_{n=2}^{\infty} \frac{F(-\infty)}{n \sin n\theta} \quad (11) \]

The curves of figs. 5-20 were obtained by putting numerical values in the expression (11) above. The parameters for each particular case are given in the accompanying curves.

I am indebted to Dr. Kirkby and the computing section of the Aerodynamics Department for the valuable aid given in the computation of the numerical examples.

Conclusions

1) The distribution of shear around the frame is not sensitive to variations in values of \( k_x \); the results show a maximum increase of 7% in value of \( N_{x0} \) at \( 30^\circ \) (i.e. maximum value) for a 100% increase in value of \( k_x \), see figs. 15, 16 and 17.

2) The distribution of shear around the frame is not sensitive to variations in skin thickness \( t \). In aircraft structures going from one gauge to the next is approximately an increase of 30% and for this increase in skin thickness the average percentage decrease in maximum value of \( N_{x0} \) is 4%. This is assuming that \( k_x \) remains constant.

3) The distribution of shear is not sensitive for reasonably large variations in the moment of inertia of the adjacent frames, and to take a mean value of \( I_f \) is sufficiently accurate for the results. The effect can be obtained from fig. (5) where it is seen that increasing the stiffness of the adjacent frames, the loaded frame remaining constant increases the maximum value of \( N_{x0} \). For the parameters chosen it is seen that the average percentage increase in value of \( N_{x0} \) at \( 30^\circ \) is 0.12% for a 1.0% increase in value of \( I_f \).

4) The distribution of shear is not sensitive for reasonably large variations in the moment of inertia of the loaded frame, and a mean value is sufficiently accurate for the results. This effect can be obtained from figs. 12, 13, 14, 18, 19 and 20.

5) The distribution of shear is not sensitive for moderate variations in frame spacing. For a frame radius up to 40" the average increase in value of \( N_{x0} \) at \( 30^\circ \) is 0.4% for a 1.0% increase in value of \( \text{"m"} \) and for a radius of 60" the increase in value of \( N_{x0} \) at \( 30^\circ \) is 0.22% for a 1.0% increase in \( \text{"m"} \). This effect can be obtained from figs. 18, 19, and 20.

6) The beam theory distribution of shear is sufficiently accurate if the value of \( r_f \) lies above the line of fig. 21. This curve is obtained by extrapolation of the curves figs. 12, 13 and 14, and is intended to give the lowest value of \( r_f \) for which the beam theory is a reasonable approximation.
7) The shear loading on frames adjacent to the loaded frame can be obtained with sufficient accuracy for practical purposes by interpolation. The shear distribution on a frame $\frac{1}{2}$ frame spacings distant from the loaded frame can be taken as that given by the beam theory and intermediate frames can be obtained by a straight line interpolation between the frames, see Fig. 4.

Reference

Derivation of the shear load due to tangential load and moment from the radial load expression.

Goodey in his work indicates a method by which the shear load for a radial load and moment may be obtained from the expression for a tangentially applied load. Here the complete solution is given for obtaining the shear load due to a tangential load and moment from the radial load expressions.

Let \((N_{\theta}x_0)\) = shear load due to a tangentially applied load \(T\).

This will be a function of \(T\) and \(\theta\).

\[(N_{\theta}x_0) = -Tf(\theta-\alpha) + Taf'(\theta)\]

when \(\alpha\) is small

\[T_\alpha = W\quad \text{and} \quad T = T \cos \alpha.\]

\[\therefore \int (N_{\theta}x_0) = Taf'(\theta) = Tf'(\theta) = (N_{\theta}x_0)'_w\]

\[= \frac{d}{d\theta} \frac{W}{T} (N_{\theta}x_0).\]

\[(N_{\theta}x_0) = \left. (N_{\theta}x_0)'_w \right|_\theta = 0 + \left. \frac{W}{T} \right|_\theta = 0 (N_{\theta}x_0)'_w \theta = 0 \quad \text{---(12)}\]

where the term \(\left. (N_{\theta}x_0)'_w \right|_\theta = 0\) is a constant of integration

and can be evaluated by considering the equilibrium of the frame.

\[-TR = \int_0^{2\pi} (N_{\theta}x_0) R^2 d\theta.\]

\[= \int_0^{2\pi} \left( \left. (N_{\theta}x_0)'_w \right|_\theta = 0 + \frac{W}{T} \right|_\theta = 0 (N_{\theta}x_0)''_w \right) R^2 d\theta.\]

\[= 2\pi R^2 \left[ (N_{\theta}x_0)''_w \right]_\theta = 0 + \frac{T}{W} \int_0^{2\pi} (2\pi - \theta)(N_{\theta}x_0)''_w \theta d\phi d\theta.\]
\[ \theta \cdot (N_{x \theta})^\dagger \theta = \frac{2\pi}{2\pi R} - \frac{T}{2\pi W} \int_0^{2\pi} (N_{x \theta})_W \varphi \, d\varphi \] which gives the constant of integration for (12) above.

\[ (N_{x \theta})_W = \frac{W}{2\pi R} \int_0^{2\pi} (N_{x \theta})_W \varphi \, d\varphi + \frac{T}{2\pi W} \int_0^{2\pi} (N_{x \theta})_W \, d\theta \quad -(13) \]

For the case considered, of a loaded frame along a cylinder

\[ (N_{x \theta})_W = \frac{W}{2\pi R} \int_0^{2\pi} (N_{x \theta})_W \varphi \, d\varphi + \frac{T}{2\pi W} \int_0^{2\pi} (N_{x \theta})_W \, d\theta \]

\[ (N_{x \theta})_W = W \sin \theta + \frac{T}{2\pi R} \int_0^{2\pi} (N_{x \theta})_W \varphi \, d\varphi \]

\[ (N_{x \theta})_W = \frac{W}{2\pi R} \int_0^{2\pi} (N_{x \theta})_W \varphi \, d\varphi + \frac{T}{2\pi W} \int_0^{2\pi} (N_{x \theta})_W \, d\theta \]

substituting in equation (13)
Let \((N_{x\theta})\) = shear load due to an applied moment.

\[
(N_{x\theta}) = \frac{M}{R} \cdot f(\theta) + \frac{\partial^2 f(\theta)}{\partial \theta^2},
\]

where \(f(\theta)\) is the shear due to unit tangential load.

\[f(\theta) = \frac{1}{\pi R} \left\{ \frac{1}{2} - \cos \theta + \sum_{n=2}^{\infty} \frac{6\pi^2}{n^2} F(---) \cos n\theta \right\}, \quad \text{see (14) above}
\]

\[f'(\theta) = \frac{1}{\pi R} \left\{ \sin \theta + \sum_{n=2}^{\infty} \frac{6\pi^2}{n^2} F(---) \cos n\theta \right\},
\]

\[
\frac{d^2 f(\theta)}{d\theta^2} = \frac{1}{\pi R} \left\{ \cos \theta + \sum_{n=2}^{\infty} \frac{6\pi^2}{n^2} F(---) n^2 \cos n\theta \right\},
\]

\[
(N_{x\theta}) = \frac{M}{R} \left\{ \frac{1}{2\pi R} - \cos \theta + \sum_{n=2}^{\infty} \frac{6\pi^2}{n^2} F(---) \cos n\theta \right\}
+ \frac{\cos \theta}{\pi R} + \sum_{n=2}^{\infty} \frac{6\pi^2}{n^2} F(---) n^2 \cos n\theta
\]

\[
= \frac{M}{\pi R} \left\{ \frac{1}{2} - \sum_{n=2}^{\infty} \frac{6\pi^2}{n^2} (1-n^2) F(---) \cos n\theta \right\},
\]
VALUES OF SHEAR/INS FOR VARYING STIFFNESS OF THE
ADJACENT FRAME, THE LOADED FRAME HAVING CONSTANT STIFFNESS.
RADIAL APPLIED LOAD OF 1000 IBS.

\[ r^4 = 43.43 \text{ in}^4 \]
\[ m = 1.62 \]
\[ R = 24.25 \text{ in} \]
\[ K_m = 1.53 \]
\[ t = 0.028 \text{ in} \]

- If = 0.05 in\(^4\), r = 8.686
- If = 0.10, r = 4.343
- If = 0.20, r = 2.172
- If = 0.40, r = 1.086
VALUES OF SHEAR/INS FOR VARYING RADII OF FRAMES.
FRAME SPACING CONSTANT AT 15 INS.
RADIAL APPLIED LOAD OF 1000 lb.

$I_f = 0.05 \text{ INS}^4$
$r = 8.666$
$k_x = 1.53$
t = 0.028 INS.

○ $R = 16.2 \text{ INS } m = 1.08$
+ $R = 24.25 \text{ INS } m = 1.62$
⊙ $R = 40 \text{ INS } m = 2.67$
△ $R = 60 \text{ INS } m = 4$. 
VALUES OF SHEAR/INS FOR VARYING RADII OF FRAMES AND CONSTANT FRAME SPACING.
RADIAL APPLIED LOAD OF 1000 LB

\[ \begin{align*}
L &= 150 \text{ INS} \\
I_f &= 0.5 \text{ INS}^4 \\
r &= 10 \\
k_x &= 153 \\
t &= 0.028 \text{ INS}
\end{align*} \]

- R = 162 INS \( m = 1.08 \)
- R = 40 INS \( m = 2.67 \)
- R = 60 INS \( m = 4.0 \)
VALUES OF SHEAR/INS FOR VARYING RADII OF FRAMES AND FRAME SPACING.
RADIAL APPLIED LOAD OF 1000 lb

\[ \frac{wL}{I} = 1.62 \]
\[ I_f = 1946 \text{ ins}^4 \]
\[ \mu = 1.0 \]
\[ k_x = 153 \]
\[ t = 0.28 \text{ ins} \]

- • \( R = 16.2 \text{ ins} \)
- + \( R = 40 \text{ ins} \)
- o \( R = 60 \text{ ins} \)
VALUES OF SHEAR/INS FOR VARYING RADIUS OF FRAMES AND FRAME SPACING.
RADIAL APPLIED LOAD OF 1000 LB.

\[ m = 2.0 \]
\[ I_f = 1946 \text{ in}^4 \]
\[ r = 3.892 \]
\[ k_x = 153 \]
\[ t = 0.020 \text{ in} \]

\[ . R = 16.2 \text{ in} \]
\[ + R = 24.25 \text{ in} \]
\[ \Delta R = 40 \text{ in} \]
\[ \bigcirc R = 60 \text{ in} \]
VALUES OF SHEAR/INS FOR VARYING RADII OF FRAMES
RADIAL APPLIED LOAD OF 1000 lb.

\[ m = 20 \]
\[ r = 25 \]
\[ k_x = 153 \]
\[ t = 0.025 \text{ ins} \]

\[ I_f = 1946 \text{ ins}^4 \quad R = 40 \text{ ins} \]
\[ I_f = 1946 \text{ ins}^4 \quad R = 60 \text{ ins} \]
VALUES OF SHEAR/INS FOR VARYING STIFFNESS OF LOADED FRAME, FRAME RADIUS CONSTANT.
RADIAL APPLIED LOAD OF 1000 LBS.

\[ I_f = 0.06 \text{ in}^4 \]
\[ R = 20 \text{ in} \]
\[ m = 2.0 \]
\[ k_x = 1.53 \]
\[ t = 0.028 \text{ in} \]

\[ r = 25 \quad rI_f = 125 \text{ in}^4 \]
\[ r = 50 \quad rI_f = 25 \text{ in}^4 \]
\[ r = 100 \quad rI_f = 50 \text{ in}^4 \]

\( \Delta = \text{BEAM THEORY} \)
VALUES OF SHEAR/INS & VARY STIFFNESS OF LOADED FRAME RADIAL APPLIED LOAD OF 1000 LB

- $R = 25$ in
- $R = 50$ in
- $R = 100$ in
- $R = 250$ in
- $R = 500$ in

- $M_F = 125$ in-lb
- $M_F = 250$ in-lb
- $M_F = 500$ in-lb

- $L = 155$ in
- $L = 310$ in
- $L = 620$ in
- $L = 1250$ in

- $t = 0.025$ in
- $t = 0.05$ in
- $t = 0.1$ in
- $t = 0.2$ in

FIG. 13.
VALUES OF SHEAR/IN. FOR VARYING STIFFNESS OF RADIAL APPLIED LOAD OF 1000 LB.

$F = 0.05$ NS.
$R = 60$ NS.
$M = 20$
$K = 1.25$
$N = 0.028$ NS.

FIG. 14.
VALUES OF SHEAR/INS FOR VARIATIONS IN VALUES OF $k_x$
RADIAL APPLIED LOAD OF 1000 lb.

<table>
<thead>
<tr>
<th>RADIUS</th>
<th>60 INS.</th>
</tr>
</thead>
<tbody>
<tr>
<td>$m$</td>
<td>20</td>
</tr>
<tr>
<td>$I_f$</td>
<td>1946 INS$^4$</td>
</tr>
<tr>
<td>$r$</td>
<td>3.892</td>
</tr>
<tr>
<td>$t$</td>
<td>0.028 INS</td>
</tr>
</tbody>
</table>

- $R = 60$ INS $k_x = 0.5$
- $R = 60$ INS $k_x = 1.0$
- $R = 60$ INS $k_x = 2.0$
VARrATtON-3
IN VALUES OF N'_x AT @ 30° FOR
VARYING VALUES OF m'I_i, m & I_i.
RADIAL APPLIED LOAD OF 1000 lb

k_x = 1.53

R = 60 INS.

R = 60 INS.

m = 162
m = 20
m = 4.0

I_f = 1946 INS^4
I_f = 0.8 INS^4

FIG 20.

VALUES OF THE MOMENT OF INERTIA OF THE
LOADED FRAME (m'I_f) ABOVE WHICH, THE
BEAM THEORY DISTRIBUTION OF SHEAR
GIVES A REASONABLE DEGREE OF ACCURACY.

FIG 21.